

APPENDIX

GENERAL STRUCTURAL FILL RECOMMENDATIONS

1. GENERAL DESCRIPTION

This item of work includes clearing and grubbing, preparation of land to be filled, filling, spreading, compaction, and control of the fill, and subsidiary work necessary to complete the grading of areas to be filled to conform with the lines, grades, and slopes as shown on the grading plans. The body of the report shall be reviewed for additional recommendations regarding the project grading.

2. CLEARING, GRUBBING, AND PREPARING AREAS TO BE FILLED

- A. All timber, logs, trees, brush, and other rubbish shall be removed, stockpiled, chipped, or otherwise disposed of so as to leave the work areas that have been disturbed with a neat and finished appearance free from unsightly debris.
- B. All vegetable matter, manure, waste/debris piles and other unsuitable material, shall be removed from the surface upon which the fill is to be placed. Subexcavate existing fill as discussed in the body of the report.
- C. Where fills are made on hillsides or slopes, the slope of the original ground upon which the fill is to be placed shall be plowed or scarified, moisture conditioned, and compacted to at least 90 percent per ASTM D1557, **unless stated otherwise in the body of the report.** Where the slope ratio of the original ground is steeper than 5 horizontal to 1 vertical, the fill shall be benched into the bank or hillside as the fill is placed and compacted in horizontal layers. The body of the report shall be reviewed for any additional requirements regarding the placement of fills on slopes.
- D. Prior to placing fill in any area, grading shall be performed as required to provide for drainage. Ditching or filling around the area shall be performed to intercept or divert surface water away from the project site.

- E. After the foundation for the fill has been cleared and plowed or scarified, it shall be disced or bladed until it is uniform and free from large clods, brought to the proper moisture content, and compacted to a depth of 6 inches and to at least 90 percent of maximum density in accordance with ASTM D1557, **unless specified otherwise in this report.**

3. **MATERIALS**

Materials for the fills shall consist of approved soil as discussed in the body of the report. If other additional fill soil is needed, an engineer from Shaw Engineering (SHAW) shall be notified at least ten working days in advance as to the source of the fill soil and a five gallon sample of the proposed fill shall be delivered to an approved materials testing laboratory for qualification tests to determine its suitability. The costs for qualification tests are the Contractor's responsibility. Any proposed fill soil should be reviewed and approved by an engineer from SHAW prior to importing the soil to the site. The soil used shall be free from vegetable matter, and other deleterious substances and the body of this report shall be reviewed for additional requirements for using imported soil as structural fill.

4. **PLACING, SPREADING, AND COMPACTING FILL MATERIAL**

- A. The fill material shall be placed in layers not to exceed 12 inches in loose thickness. Each layer shall be spread evenly and shall be thoroughly mixed during spreading to ensure uniformity of the material in each layer. Layer placement shall be so controlled that no ponding of water can occur on any working surface.
- B. When the moisture content of the fill material is below that required to achieve the specified density, water shall be added to each layer by sprinkling; the soil shall be disced or otherwise manipulated to assure uniform distribution of moisture until the moisture content is satisfactory.
- C. When the moisture content of the fill material is greater than that required, the fill material shall be aerated by scarifying until the moisture content is satisfactory.

- D. After each layer has been placed, mixed, and spread evenly, it shall be thoroughly compacted to at least 95 percent of maximum density in accordance with ASTM D1557, unless specified otherwise in this report. Compaction shall be by means of tamping, sheepsfoot rollers, vibratory smooth drum rollers or other equipment suitable for the type of soil to be compacted. Compaction shall be accomplished while the fill soil is at the required moisture content. Compaction of each layer shall be continuous over its entire area, and the compaction equipment shall make sufficient trips to ensure that the desired density has been obtained.
- E. Field density tests shall be made by an engineer from SHAW, his representative or an approved testing laboratory after the compacting of each layer of fill. No additional layers of fill shall be spread until the field density tests indicate that the specified density has been obtained. If the percent compaction is not satisfactory, the Contractor will be required to adjust the soil moisture content, increase the weight of the compaction equipment and/or the number of passes, and/or decrease the lift thickness as required to achieve the specified density.

5. **OBSERVATION**

Continuous observation by an Engineer from SHAW, his representative or an approved testing laboratory shall be made during filling and compacting operations. The Project Owner or Contractor shall give at least 48 hours notice prior to such operations to allow scheduling of the work.

6. **SEASONAL LIMITS**

No fill material shall be placed, spread, or rolled during unfavorable weather conditions. During periods of unfavorable weather conditions, the surface of the fill may require being sealed by rolling with a smooth drum roller to help prevent excessive moisture from entering or leaving the fill. When the work is interrupted by precipitation or freezing temperatures, fill operations shall not be resumed until the field density tests indicate that the moisture content and density of the previously placed fill are satisfactory.

SHAW ENGINEERING, LTD.

Western Environmental Testing Laboratory

Report Comments

Shaw Engineering - 1409665

General Comments

None

Specific Comments

None

Per method recommendation (section 4.4), Samples analyzed by methods EPA 300.0 and EPA 300.1 have been filtered prior to analysis.

Report Legend

- B -- Blank contamination; Analyte detected above the method reporting limit in an associated blank
- HT -- Sample analyzed beyond the accepted holding time
- J -- The reported value is between the laboratory method detection limit and the laboratory practical quantitation limit
- M -- Reported value is estimated; The sample matrix interfered with the analysis
- N -- There was insufficient sample available to perform a spike and/or duplicate on this analytical batch.
- NC -- Not calculated due to matrix interference
- Q -- Reported value is estimated; The value failed to meet QC criteria for either precision or accuracy
- S -- Surrogate recovery was outside of laboratory acceptance limits due to matrix interference. The associated blank and LCS surrogate recovery was within acceptance limits
- SC -- Spike recovery not calculated. Sample concentration >4X the spike amount; therefore, the spike could not be adequately recovered

SPARKS

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EPA LAB ID: NV00925 - ELAP No: 2523

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LAS VEGAS

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Western Environmental Testing Laboratory

Analytical Report

Shaw Engineering

20 Vine Street

Reno, NV 89503

Attn: Dennis Keely

Phone: (775) 329-5559 Fax: (775) 329-5406

PO\Project: 14026.10D / TMWA Sutro #2

Date Printed: 10/3/2014

OrderID: 1409665

Customer Sample ID: TP-1@ 4.5' to 6.0'

Collect Date/Time: 9/23/2014

WETLAB Sample ID: 1409665-001

Receive Date: 9/24/2014 08:50

Analyte	Method	Results	Units	DF	RL	Analyzed	LabID
General Chemistry							
Redox Potential	SM 2580B	360	mV	1		10/1/2014	NV00925
Paste pH	SW846 9045B	8.27	pH Units	1		9/25/2014	NV00925
Resistivity	SM 2510B	1700	ohms.cm	1	1.0	10/2/2014	NV00925
Anions by Ion Chromatography							
Chloride	EPA 300.0	ND	mg/kg	15	15	9/26/2014	NV00925
Sulfate	EPA 300.0	ND	mg/kg	15	15	9/26/2014	NV00925
Sample Preparation							
Saturated Paste Preparation	CSTPM S:1.0	Complete		1		9/24/2014	NV00925
3:1 DI Water Extraction	WL 3.0	Complete		1		9/24/2014	NV00925

Customer Sample ID: TP-2@ 1.5' to 3.8'

Collect Date/Time: 9/23/2014

WETLAB Sample ID: 1409665-002

Receive Date: 9/24/2014 08:50

Analyte	Method	Results	Units	DF	RL	Analyzed	LabID
General Chemistry							
Redox Potential	SM 2580B	390	mV	1		10/1/2014	NV00925
Paste pH	SW846 9045B	6.92	pH Units	1		9/25/2014	NV00925
Resistivity	SM 2510B	570	ohms.cm	1	1.0	10/2/2014	NV00925
Anions by Ion Chromatography							
Chloride	EPA 300.0	38	mg/kg	15	15	9/26/2014	NV00925
Sulfate	EPA 300.0	49	mg/kg	15	15	9/26/2014	NV00925
Sample Preparation							
Saturated Paste Preparation	CSTPM S:1.0	Complete		1		9/24/2014	NV00925
3:1 DI Water Extraction	WL 3.0	Complete		1		9/24/2014	NV00925

DF=Dilution Factor, RL=Reporting Limit, ND=Not Detected or <RL

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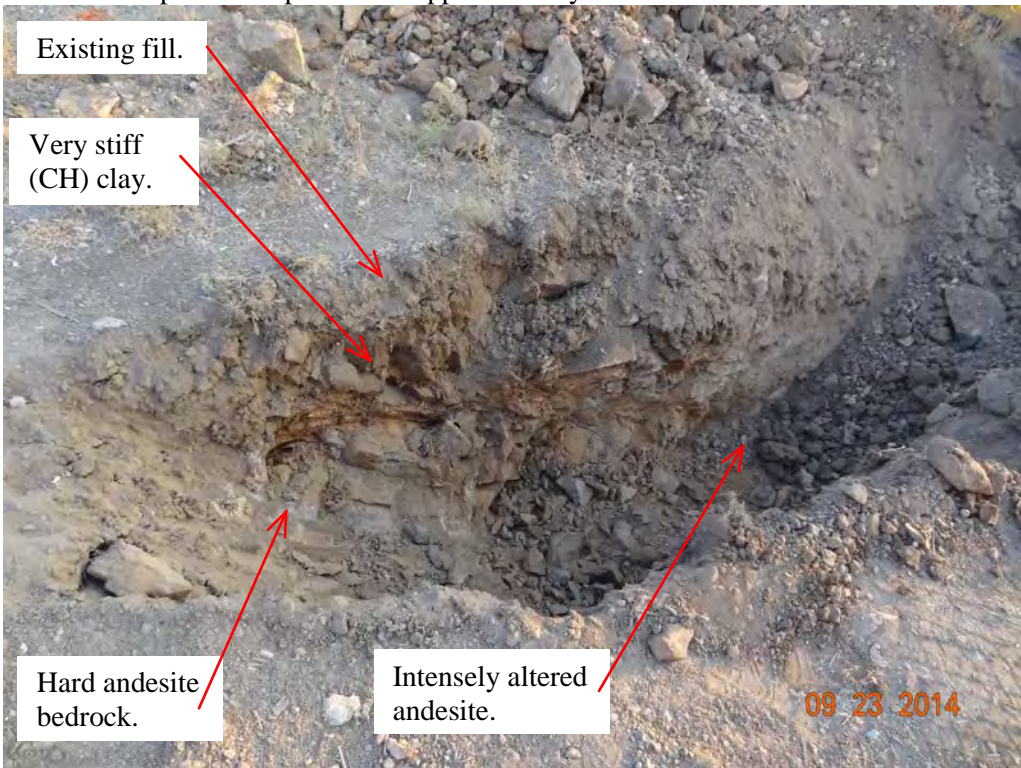
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Field resistivity testing in the approximate east-west alignment. White painted lath with white ribbons are the approximate building corners.



Test pit TP-1. Looking at soft to hard, andesite bedrock that is moderately to intensely hydrothermally altered. Excavation depth in this photo is at approximately 4 feet.



Looking westerly at test pit TP-1 location just north of the proposed building.

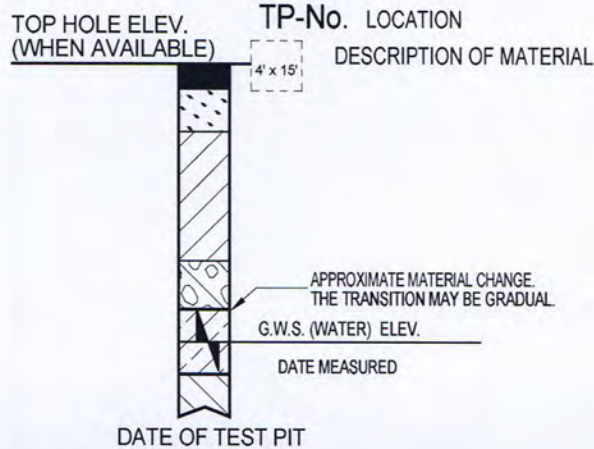


Looking southwesterly at test pit TP-2.



LEGEND OF EXCAVATING, SAMPLING AND TESTING

TEST PIT



Note:
 The Log of Test Pits and Log of Borings show subsurface conditions at the dates and locations indicated, and it is not warranted that they are representative of subsurface conditions at other locations. The passage of time, changed climatic conditions and seasonal effects may result in conditions that differ from those depicted by the logs. The stratification lines represent the approximate boundary between soil types or relative densities and the transition may be gradual.

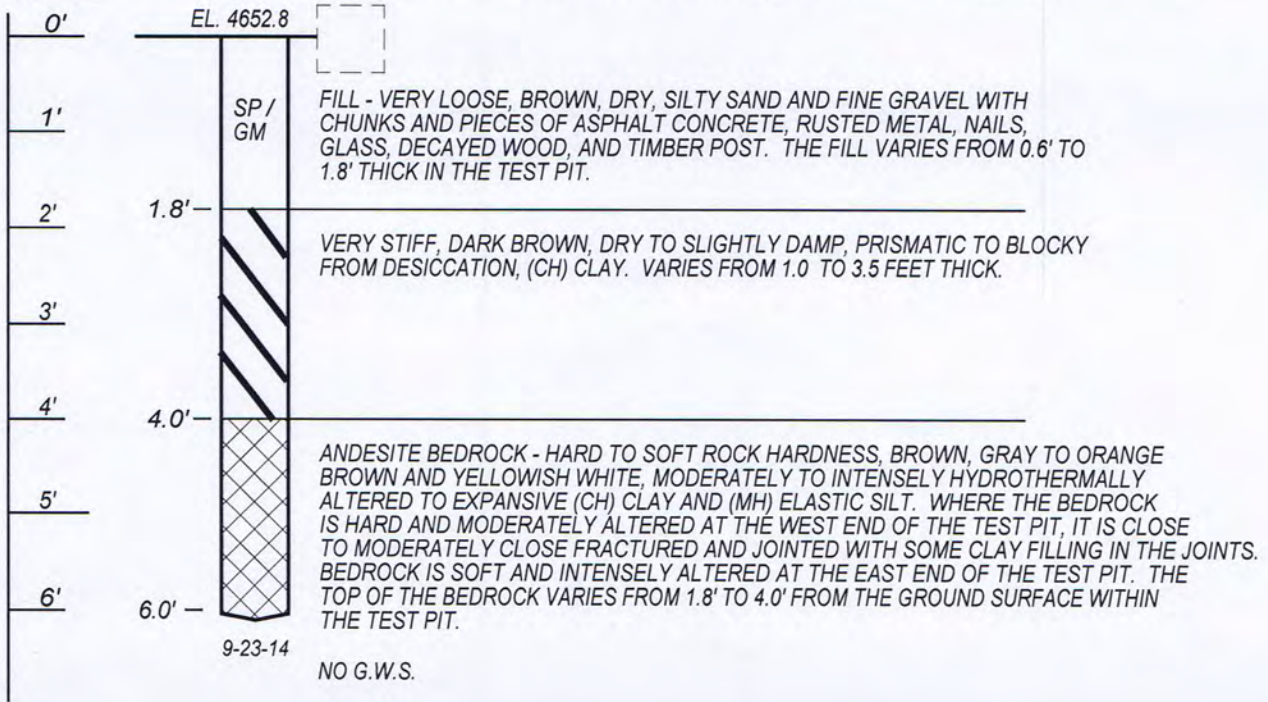
ROCK CLASSIFICATION		SOIL CONSISTENCY CLASSIFICATION		
SYMBOL	NAME	CONSISTENCY		* BLOWS PER FOOT
[Cross-hatch pattern]	IGNEOUS ROCK	GRANULAR	COHESIVE	
		VERY LOOSE	VERY SOFT	
[Horizontal lines]	SEDIMENTARY ROCK	LOOSE	SOFT	5 TO 10
		SLIGHTLY COMPACT	STIFF	10 TO 20
[Wavy lines]	METAMORPHIC ROCK	COMPACT	VERY STIFF	20 TO 35
		DENSE	HARD	35 TO 70
		VERY DENSE	VERY HARD	+70
NOTE: CLASSIFICATION OF EARTH MATERIALS SHOWN ON THE LOG OF TEST PITS ARE BASED UPON FIELD INSPECTION UNLESS NOTED OTHERWISE.		*(STANDARD PENETRATION TEST) BLOWS PER FOOT (140 LB. HAMMER 30" FREE-FALL BLOW USING 2" O.D. x 1 3/8" I.D. SAMPLER).		

SYMBOLS		THE UNIFIED SOIL CLASSIFICATION SYSTEM							
		MAJ. DIV.	LETTER	SYMBOL	NAME	MAJ. DIV.	LETTER	SYMBOL	NAME
FLUSH-COUPLED PENETROMETER 2 1/4" CONE PENETROMETER SAMPLER BORING (DRY) ROTARY BORING (WET)	AUGER BORING (DRY) JET BORING DIAMOND CORE BORING. TEST PIT (SIZE) W x L = Approximate Width and Length	COARSE GRAINED SOILS GRAVEL AND GRAVELLY SOIL SAND AND SANDY SOIL	GW	[Symbol]	WELL-GRADED GRAVEL OR GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	FINE GRAINED SOILS SILTS AND CLAYS, LL<50 SILTS AND CLAYS, LL>50 HIGHLY ORGANIC SOILS	ML	[Symbol]	INORGANIC SILT AND VERY FINE SAND, ROCK FLOUR, SILTY OR CLAYEY FINE SAND OR CLAYEY SILT WITH SLIGHT PLASTICITY.
	GP		[Symbol]	POORLY-GRADED GRAVEL OR GRAVEL-SAND MIXTURES, LITTLE OR NO FINES.	CL		[Symbol]	INORGANIC CLAY OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAY, SANDY CLAY SILTY CLAY, LEAN CLAY	
GM	[Symbol]		SILTY GRAVEL, GRAVEL-SAND-SILT MIXTURES.	OL	[Symbol]		ORGANIC SILT AND ORGANIC SILT-CLAY OR LOW PLASTICITY.		
GC	[Symbol]		CLAYEY GRAVEL, GRAVEL-SAND-CLAY MIXTURES.	MH	[Symbol]		INORGANIC SILT, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS, ELASTIC SILT.		
SW	[Symbol]		WELL-GRADED SAND OR GRAVELLY SAND, LITTLE OR NO FINES.	CH	[Symbol]		INORGANIC CLAY OF HIGH PLASTICITY, FAT CLAY		
SP	[Symbol]		POORLY-GRADED SAND OR GRAVELLY SAND, LITTLE OR NO FINES.	OH	[Symbol]		ORGANIC CLAY OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILT.		
SM	[Symbol]		SILTY SAND, SAND-SILT MIXTURES	PT	[Symbol]		PEAT AND OTHER HIGHLY ORGANIC SOILS.		
SC	[Symbol]		CLAYEY SAND, SAND-CLAY MIXTURES						

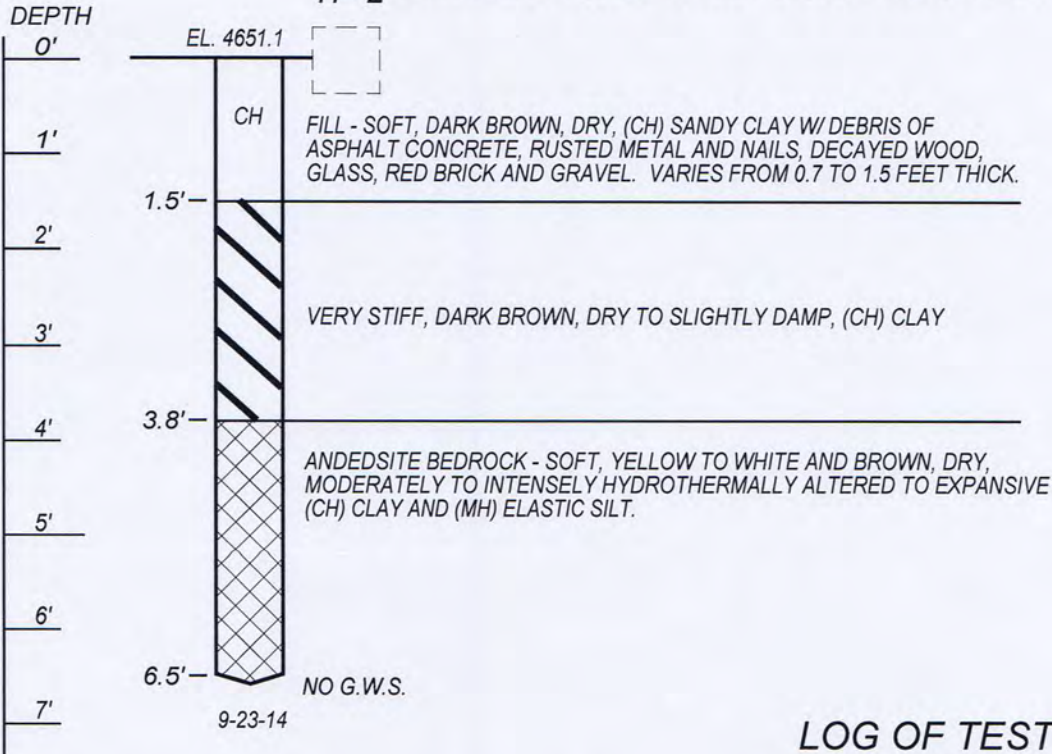
SHAW ENGINEERING, LTD. RENO, NEVADA LEADERS IN ESSENTIAL ENGINEERING EXPERTISE	TRUCKEE MEADOWS WATER AUTHORITY SUTRO #2 PUMP STATION RENO, NEVADA	FIGURE 1
Job No: 14026.10D Appr: DK Date: 9-23-14		

DEPTH

TP-1 APPROXIMATELY 10 FEET NORTH OF THE PROPOSED BUILDING



TP-2 APPROXIMATELY 10 FEET SOUTH OF THE PROPOSED BUILDING



LOG OF TEST PITS

VERTICAL SCALE: 1" = 2'

NOTE:
TOP HOLE ELEVATIONS WERE SURVEYED FROM A CONTROL POINT LABELED #102 ON SELMI DRIVE JUST NORTH OF THE SITE. THE REPORTED ELEVATION OF THIS CONTROL POINT IS 4655.77 FEET.

NOTE:
SEE FIGURE 1 FOR LEGEND OF EXCAVATING, SAMPLING, AND TESTING.

SHAW ENGINEERING, LTD.
RENO, NEVADA
LEADERS IN ESSENTIAL ENGINEERING EXPERTISE

TRUCKEE MEADOWS WATER AUTHORITY
SUTRO #2 PUMP STATION
RENO, NEVADA

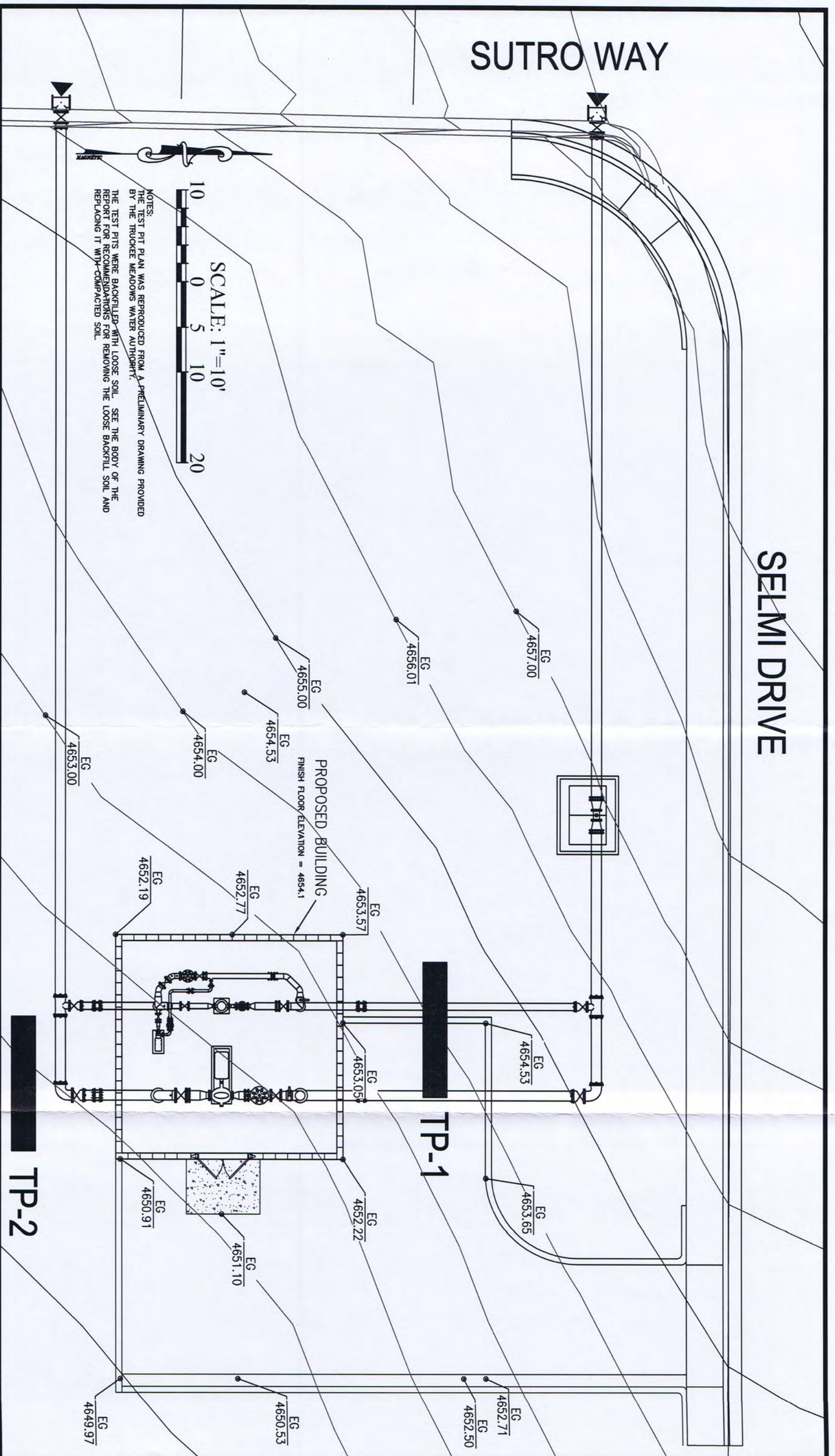
FIGURE

2

Job No.: 14026.10D Appr.: DK Date.: 9-23-14

SELMI DRIVE

SUTRO WAY



NOTES:
 THE TEST PIT PLAN WAS REPRODUCED FROM A PRELIMINARY DRAWING PROVIDED BY THE TRUCKEE MEADOWS WATER AUTHORITY.
 THE TEST PITS WERE BACKFILLED WITH LOOSE SOIL. SEE THE BODY OF THE REPORT FOR RECOMMENDATIONS FOR REMOVING THE LOOSE BACKFILL SOIL AND REPLACING IT WITH COMPACTED SOIL.

SCALE: 1"=10'

PROPOSED BUILDING
 FINISH FLOOR ELEVATION = 4654.1

TP-1

TP-2



TRUCKEE MEADOWS WATER AUTHORITY
 SUTRO #2 PUMP STATION
 RENO, NEVADA

TEST PIT PLAN

Figure 3
 14026.10D
 October 10, 2014