GEOTECHNICAL INVESTIGATION

Marsh-Booth 24 Inch Water Main Project Reno, Nevada

December 6, 2017



1355 Capital Boulevard Reno, Nevada 89502



ESE Project No. 17.1.30

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Prepared by:



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1.0 INTRODUCTION AND SCOPE

This Report presents the results of Eastern Sierra Engineering's (ESE's) geotechnical investigation for the proposed Marsh-Booth 24-inch Water Main project in Reno, Nevada. The project area includes 1,055 +/- feet on Marsh Avenue, 645 +/- feet on California Avenue, 1,545 +/- feet on Booth Street and 490 +/- feet on a private roadway leading from Booth Street to the Idlewild Pump Station in Reno, Nevada. We understand that the depth to invert may be as deep as 12 feet below finish grade, and possibly 20 feet due to possible conflicts with existing underground utilities. The limits of the project are shown on Figure 1.

The following scope of services was provided:

- Located and marked seven (7) boring locations and contacted Underground Service Alert (USA) to notify them of our intent to excavate and drill at the site;
- Drilled, logged, and sampled six (6) borings ranging in depth from 15 feet to 20 feet;
- Performed laboratory testing that included particle size analysis, Atterberg limits, moisture content, and corrosivity (pH, sulfate, chloride, resistivity, sulfides and redox potential);
- Prepared conclusions and recommendations addressing excavatability of subsurface soils, reuse of excavated material for bedding and backfill material, and mitigation of unstable subgrade;
- Prepared this report that summarizes field activities, subsurface soil conditions and laboratory testing. Also included is a site plan with approximate boring locations, boring logs, particle size distribution charts, plasticity charts, and construction recommendations.

This report was prepared for the sole use of the Truckee Meadows Water Authority (TMWA), the only intended beneficiary of ESE's work. No other party should rely on the information contained herein without prior written consent of TMWA and ESE.



2.0 FIELD INVESTIGATION AND LABORATORY TESTING

2.1 Field Investigation

ESE explored subsurface conditions by drilling six (6) borings (B-1 through B-6) on October 16 and 17, 2017. Borings were drilled to depths ranging from 15 to 20 feet below the existing grade using a track mounted sonic drill rig. A sonic rig drill advances by rotating and vibrating the rod, core barrel, and casing at sonic frequencies. The boring is drilled, cored and cased at the same time while producing a continuous core sample from the entire depth of the hole. The approximate locations of borings are presented on Figure 1. During drilling activities, ESE's geologist logged the borings and obtained continuous representative soil samples from the borings.

During drilling, existing asphalt concrete (AC) pavement and aggregate base (AB)/fill thicknesses (if encountered) were measured. At the completion of logging and sampling, the borings were backfilled and compacted with the excavated material and completed to the original grade with an asphalt concrete cold patch material. Groundwater, if encountered, was measured prior to backfilling test pits. Summary logs of borings B-1 through B-6 are presented on Figures 2 through 13.

Soil samples were classified in accordance with the Unified Soil Classification System (ASTM D2487-00) presented on Figure 14.

2.2 Laboratory Testing

Soil samples were taken to ESE's American Association of State Highway and Transportation Officials (AASHTO) certified materials testing laboratory for further examination and selected laboratory testing. Laboratory testing included particle size analysis, Atterberg limits (plasticity), moisture content; additionally select samples were sent to WETLAB for corrosivity (pH, sulfate, sulfides chloride, resistivity and redox potential) testing. Results of laboratory testing are presented in Figures 15 through 29. Results are also summarized in Table 1 and on the logs of the borings, Figures 2 through 13.



Boring No.	Sample Depth (ft.)	Moisture Content (%)	Percent Passing No. 200 Sieve Size (%)	Atterberg Limits (%)	Soil Type
B-1	4.0 - 6.0	3.0	14.1	LL = 15 PI = 2	Silty Gravel with Sand (GM)
B-1	8.0 - 9.0	2.0	9.7	LL = 16 PI = 3	Poorly Graded Gravel with Silt and Sand (GP-GM)
B-1	11.0 - 13.0	3.0	22.0	LL = 17 PI = 2	Silty Gravel with Sand (GM))
В-2	9.0 - 11.0	2.0	8.5	LL = 16 PI = 2	Poorly Graded Gravel with Silt and Sand (GP-GM)
В-3	2.0 - 4.0	3.0	9.1	LL = 21 PI = 3	Poorly Graded Gravel with Silt and Sand (GP-GM)
В-3	10.0 - 12.0	3.0	8.1	LL = PI = NP	Poorly Graded Gravel with Silt and Sand (GP-GM)
В-3	18.0 - 20.0	3.0	17.3	LL = 25 PI = 9	Clayey Gravel with Sand (GC)

TABLE 1 - Laboratory Test Results



Boring No.	Sample Depth (in.)	Moisture Content (%)	Percent Passing No. 200 Sieve Size %	Atterberg Limits (%)	Soil Type
B-4	3.0 - 5.0	8.0	15.8	LL = 29 PI = 4	Silty, Clayey Gravel with Sand (GC-GM)
B-4	9.0 – 11.0	9.0	10.6	LL = 30 PI = 1	Poorly Graded Gravel with Silt and Sand (GP-GM)
B-5	5.0-9.0	3.0	8.8	LL = PI = NP	Poorly Graded Gravel with Silt and Sand (GP-GM)
B-6	9.0 -11.0	2.0	12.5	LL= 21 PI = 2	Silty Gravel with Sand (GM)
B-6	13.0 - 15.0	4.0	11.9	LL = 21 $PI = 3$	Poorly Graded Gravel with Silt and Sand (GP-GM)

 TABLE 1 - Laboratory Test Results (cont.)

3.0 SURFACE AND SUBSURFACE CONDITIONS

3.1 Surface Conditions

The Booth Street section of the proposed alignment ranged from 6.5-inches to 11-inches of AC pavement, aggregate base/fill ranged from 4-inches to 6-inches. On California Avenue the AC was found to be 8-inches thick in eastbound right lane, overlying 10-inches of granular fill, in the left lane the AC was 10.5-inches thick, overlying 7-inches of granular fill. The Marsh Avenue section ranged from 9.5-inches to 11.5-inches of AC and the fill ranged from 4-inches to 6-inches.

3.2 Subsurface Conditions

The pipeline alignment is located within an area mapped by the Nevada Bureau of Mines and Geology (NBMG) in the Reno Folio Geologic Map, Reno, Nevada (H.F. Bonham Jr. and E. C. Bingler, 1973,) as: "Qto" – Tahoe Outwash – "Boulder to cobble gravel, sandy gravel, and gravely sand. Contains giant boulders. Rock clasts are rounded to



subrounded and, in decreasing order of abundance, are granitic, volcanic, and metamorphic." "Qdo" - Donner Lake Outwash – "Deposits similar to Tahoe outwash except weathered to depths of four feet or more".

Fill material was encountered in all the borings. Fill underlying the AC generally consisted of medium dense to dense, poorly graded gravel with sand, poorly graded sand, and silty sand ranging in thickness from 4.0-inches to 10.0-inches beneath the AC.

Based on ESE's field investigation and laboratory testing, subsurface soils underlying the fill generally consist of medium dense to very dense, poorly graded gravel with sand, cobble and boulders, poorly graded sand with gravel, cobble and boulders, silty gravel with sand, cobble and boulder, and clayey sand.

At the B-4 location on California Ave., a boring was first attempted in right lane in east bound direction, 4.5' from front face of curb, and we encountered concrete (wall footing) at 18-inches below top of finish grade. A second attempt to drill a boring was performed in left lane in east bound direction, 16.5 feet from front face of curb, and we did not encounter concrete.

No groundwater was encountered at the time of drilling activities. Groundwater elevations should be expected to seasonally fluctuate due to precipitation and snowmelt and irrigation.

4.0 CONCLUSIONS AND RECOMMENDATIONS

4.1 General

Based on the field investigation, laboratory testing, and engineering analysis, the site has no geotechnical engineering constraints.

4.2 Excavation

Based on the field exploration, excavation for the majority of the water main alignment will pass through dense to very dense gravels and cobbles. Borings encountered areas of abundant cobbles and some boulders 24 inches in diameter or greater. The contractor should anticipate encountering boulders larger than 24-inches in size along the water main alignment. Some of the borings encountered slow, difficult drilling due to cobble and boulder size rock. Drilling produced some rock cores of a foot or greater in a number of the borings where boulder size material was encountered.

Based on drill rates, rock cores and other material recovered during drilling, a large track hoe equipped with a "rock bucket and rock teeth" would most likely be able to excavate to the proposed water main invert elevations. In some locations the use of a jackhammer or similar pneumatic method may be necessary to break-up very rocky areas and thus allowing excavation.



Additionally, Contractor should pothole existing utilities to verify location and depth to ensure that adequate clearance can be maintained between new water main and existing utilities.

4.3 Water Main Bedding and Backfill

Any material used as pipe bedding or backfill shall meet the minimum requirements of the TMWA Construction and Design Standards (TMWACDS).

4.3.1 Water Main Bedding

All bedding material shall conform to the requirements of the "Standard Specification for the Public Works Construction," (Orange Book), Revision dated 12-21-2016 and referenced further in this report as the Standard Specifications or other TMWACDS approved alternate.

Class A bedding shall conform to the Standard Specifications, Section 200.03.02. The bedding area (6 inches below pipe, 12 inches laterally beyond pipe, and 12 inches over pipe) should be Class A material moisture conditioned to near optimum moisture content, placed in 8 inch maximum loose lifts, then compacted to at least 95 percent relative compaction. The bedding materials shall be placed as described in Section 305 of the Standard Specifications and as detailed on the drawings.

Based on the field investigation and laboratory testing, the excavated native material will not meet Class A bedding specifications.

4.3.2 Water Main Backfill

The backfill over the bedding material shall be per the requirements of the Drawings and Specifications for the project and shall be Type II, Class B Aggregate Base conforming to Section 200.01.03 of the Standard Specifications. Organic matter and oversized rock will not be permitted.

All backfill shall be moisture conditioned to near optimum moisture content, placed in lifts not exceeding 8 inches (loose thickness), and compacted to the requirements as provided in the Specifications and detailed on the Drawings. Backfill material shall be placed as described in Section 305 of the Standard Specifications.



4.4 Unstable Subgrade

If areas of soft, wet, unstable subgrade are encountered or created, it may require the Contractor to overexcavate and stabilize the subgrade by placing Engineer approved 8-inch to 12-inch diameter, clean, crushed, angular rock, and/or combine the crushed rock with Mirafi HP570 woven geofabric (or acceptable equivalent) to create a working platform. **NOTE:** A test area is recommended to determine the most suitable method of creating a working platform. Relatively light, nonvibratory compaction equipment shall be used during this operation to minimize further softening and pumping of the exposed subgrade.

Any groundwater encountered in excavations shall be removed and excavations kept dry until they are backfilled to at least 2 feet above the static groundwater table. Where static groundwater is within 2 feet of existing grade, excavations shall be kept dry until they are completely backfilled.

4.5 Temporary Excavations

The Contractor is responsible for the selection, design, construction and maintenance of the shoring method and temporary slopes. Safety requirements established by OSHA or other regulatory agencies shall be followed during excavation and construction by the Contractor. Heavy construction equipment, construction materials, or soil stockpiles shall not be located near the top of any excavation. Sloughing of excavation sidewalls should be anticipated during construction excavation due to cohesionless granular soils.

4.6 Corrosion Potential

The results of the corrosion testing on boring soil samples are presented in Table 2.

Boring No.	Sample Depth (ft.)	Chloride (mg/kg)	Sulfate (mg/kg)	Resistivity (ohm-cm)	рН	Sulfides	Redox potential (mV)
B-2	11.0-13.0	6.1	6.7	1200	9.3	Negative	470
B-3	10.0-12.0	5.5	4.4	9500	9.0	Negative	460
B-3	16.0-18.0	11	15	6700	8.7	Negative	400
B-4	9.0-11.0	54	35	4000	8.0	Negative	470
B-5	5.0-9.0	78	31	2400	8.7	Negative	450

TABLE - 2Corrosivity Test Results

Based on the results of analytical testing there is negligible potential for sulfate exposure/attack to concrete, therefore a Type II cement is acceptable for use. Also, based on the results of laboratory testing the degree of corrosive potential for metals varies from moderately corrosive (10,000-5,000 ohm-cm) to highly corrosive (3,000-1,000 ohm-cm).



Corrosion protection of metal pipelines shall be based upon the recommendations found in AWWA C105, Appendix A and the Design Engineers judgment.

5.0 ADDITIONAL SOILS ENGINEERING SERVICES

Prior to and during construction, the following should be performed under ESE observations to ensure conformance with the intent of ESE's recommendations.

- Excavation;
- Suitability of onsite and imported fill materials;
- Bedding and backfill placement and compaction;

Observation of these operations will allow ESE to check that soil conditions are consistent with this geotechnical investigation and to evaluate variations in soils conditions, which may require special consideration or modification of the recommendations.

6.0 LIMITATIONS

Recommendations contained in this report are based on our field explorations, laboratory tests, and our understanding of the proposed construction. The study was a cost-effective method to evaluate some of the potential geotechnical concerns.

The soils data used in the preparation of this report were obtained from borings located for this investigation. It is possible that variations in soils exist between the points explored. The nature and extent of soil variations may not be evident until construction occurs. If any soil conditions are encountered at this site, which are different from those described in this report, our firm should be immediately notified so that we may make any necessary revisions to our recommendations.

This report may be used only by TMWA and only for the purposes stated within a reasonable time from issuance, but in no event later than three years from the date of the report. Land or facility use, on and off-site conditions, regulations, or other factors may change over time, and additional work may be required with the passage of time.



7.0 **REFERENCES**

- *Geologic Map of the Reno Folio, Washoe County, Nevada*. H.F. Bonham Jr. and E.C. Bingler, Nevada Bureau of Mines and Geology, U.S. Geological Survey, 1973.
- Standard Specifications for Public Works Construction. RTC of Washoe County, Washoe County, City of Sparks, City of Reno, Carson City, and City of Yerington, Revision No. 9, December 21, 2016
- *Construction and Design Standards*, Truckee Meadows Water Authority https://tmwa.com/new-construction/standards/



APPENDIX A - FIGURES





























APPENDIX B - LABORATORY RESULTS



		MAJOR	DIVISIONS					TYPICAL NAMES
	GRAVELS		CLEAN GRAVELS WITH LITTLE OR NO FINES		GW	0.0	0.00	WELL GRADED GRAVELS WITH OR WITHOUT SAND, LITTLE OR NO FINES
SER					GP	0.00	0000	POORLY GRADED GRAVEL WITH OR WITHOUT SAND, LITTLE OR NO FINES
SOIL COAR SIEVE	MORE TH	FRACTION	GRAVELS WITH OVER	2	GM	00		SILTY GRAVELS, SILTY GRAVELS WITH SAND
AINED LF IS 200 S	No. 4 SI	EVE SIZE	12% FINES		GC	VA2	8	CLAYEY GRAVELS, CLAYEY GRAVELS WITH SAND
E-GR			CLEAN SANDS WITH	н	SW			WELL GRADED SANDS WITH OR WITHOUT GRAVEL, LITTLE OR NO FINES
OARSI RE THI	SAN	IDS	LITTLE OR NO FINES	5	SP	· · · ·	•	POORLY GRADED SANDS WITH OR WITHOUT GRAVEL, LITTLE OR NO FINES
MOR	MORE TH COARSE	FRACTION	SANDS WITH OVER		SM		•	SILTY SANDS WITH OR WITHOUT GRAVEL
	No. 4 SI	EVE SIZE	12% FINES		SC	1.		CLAYEY SANDS WITH OR WITHOUT GRAVEL
ER		CU 1			ML			INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTS WITH SANDS AND GRAVELS
SOILS S FIN SIEVE		SILIS	AND CLAYS		CL			INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, CLAYS WITH SANDS AND GRAVELS, LEAN CLAYS
NED 1ALF 200	LIQUID LIMIT 50% OR LESS				OL	• • •	•	ORGANIC SILTS OR CLAYS OF LOW PLASTICITY
-GRAI HAN F	SILTS AND CLAYS				ΜН			INORGANIC SILTS, MICACEOUS OR DIATOMACIOUS, FINE SANDY OR SILTY SOILS, ELASTIC SILTS
FINE- ORE T THAN					СН			INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS
Ŵ	LIQUID LIMIT GREATER THAN 50%				ОН	1/	1	ORGANIC SILTS OR CLAYS OF MEDIUM TO HIGH PLASTICITY
	HIGHLY ORGANIC SOILS				Pt			PEAT AND OTHER HIGHLY ORGANIC SOILS
M(80	0) –	Moisture	Content (%)		Г			Shear Strength (pst)
DD(105) -	Dry Den	sity (pcf)	TxUL) 320	۲ 00 (260)0)	——Confining Pressure — Unconsolidated Undrained Triaxial Shear
Perr	n –	Permeability			FM) d	or (S)		- (field moisture or saturated)
Con	sol –	Consolid	solidation T;			0 (260)0)	- Consolidated Undrained Triaxial Shear
LL	-	Liquid L	quid Limit (%)					- (with or without pore pressure measurement)
PI	-	– Plasticity Index (%)			320	0 (260)0)	- Consolidated Drained Triaxial Shear
Gs	-	Specific Gravity			J 320	00 (26	20)	- Simple Shear Consolidated Undrained
MA		Particle	Size Analysis	(P)			- (with or without pore pressure measurement)
OC		Organic	Organic Content			00 (260	00)	- Simple Shear Consolidated Drained
R-V	′alue —	Resistan	ce Value	DSC	D 270	00 (20	00)	- Consolidated Drained Direct Shear
CBR	-	Californi	alifornia Bearing Ratio			70		- Unconfined Compression
		"Undistu	rbed" Sample	11/5	70	0		

Bulk or Classification Sample DSUU - Unconsolidated Undrained Direct Shear

MARSH-BOOTH WATER MAIN TRUCKEE MEADOWS WATER AUTHORITY UNIFIED SOIL CLASSIFICATION

FIGURE

14

DRAWNJOB NUMBERAPPROVEDDATEREVISEDDATEMPP17.1.30SWJ11/7/2017

ERN

GINEERING

RRA



